
Improved Design of Drilled Shafts in Rock

**By: ADSC Southeast Chapter, and
Dan Brown, P.E., Ph.D., *Auburn University***

Objectives

- ◆ Improve design methodology in rock
- ◆ Improve cost-effectiveness of drilled shafts in rock in key Southeastern markets
- ◆ Demonstrate reliability of drilled shafts founded on rock

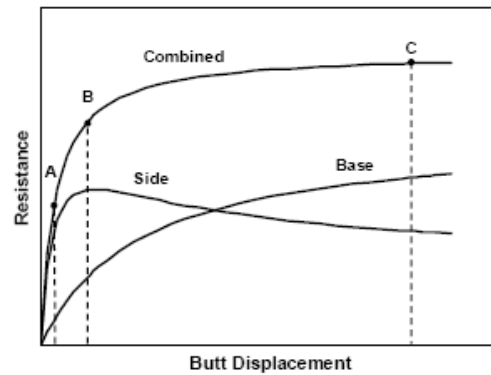
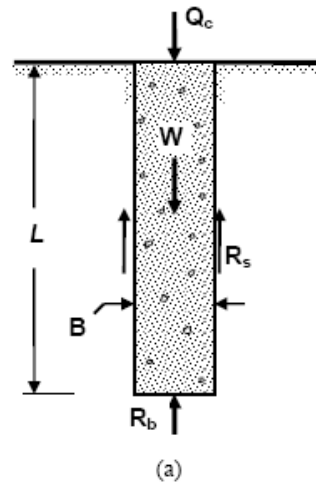


Design for Axial Loading

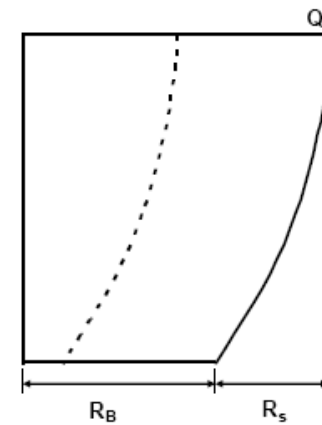
- ◆ Geotechnical Strength Limit State
 - ◆ Plunging failure
- ◆ Structural Strength Limit State
 - ◆ Structural failure
- ◆ Servicability Limit State
 - ◆ Settlements or Axial Displacement



Generalized Behavior Under Axial Load

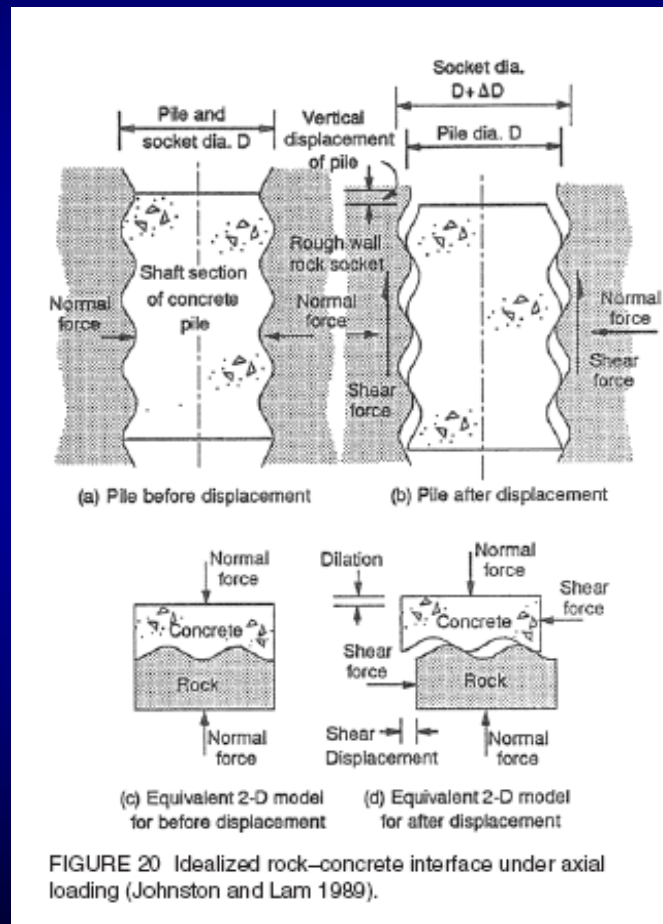


(b)



(c)

Dilation at Rock/Shaft Interface



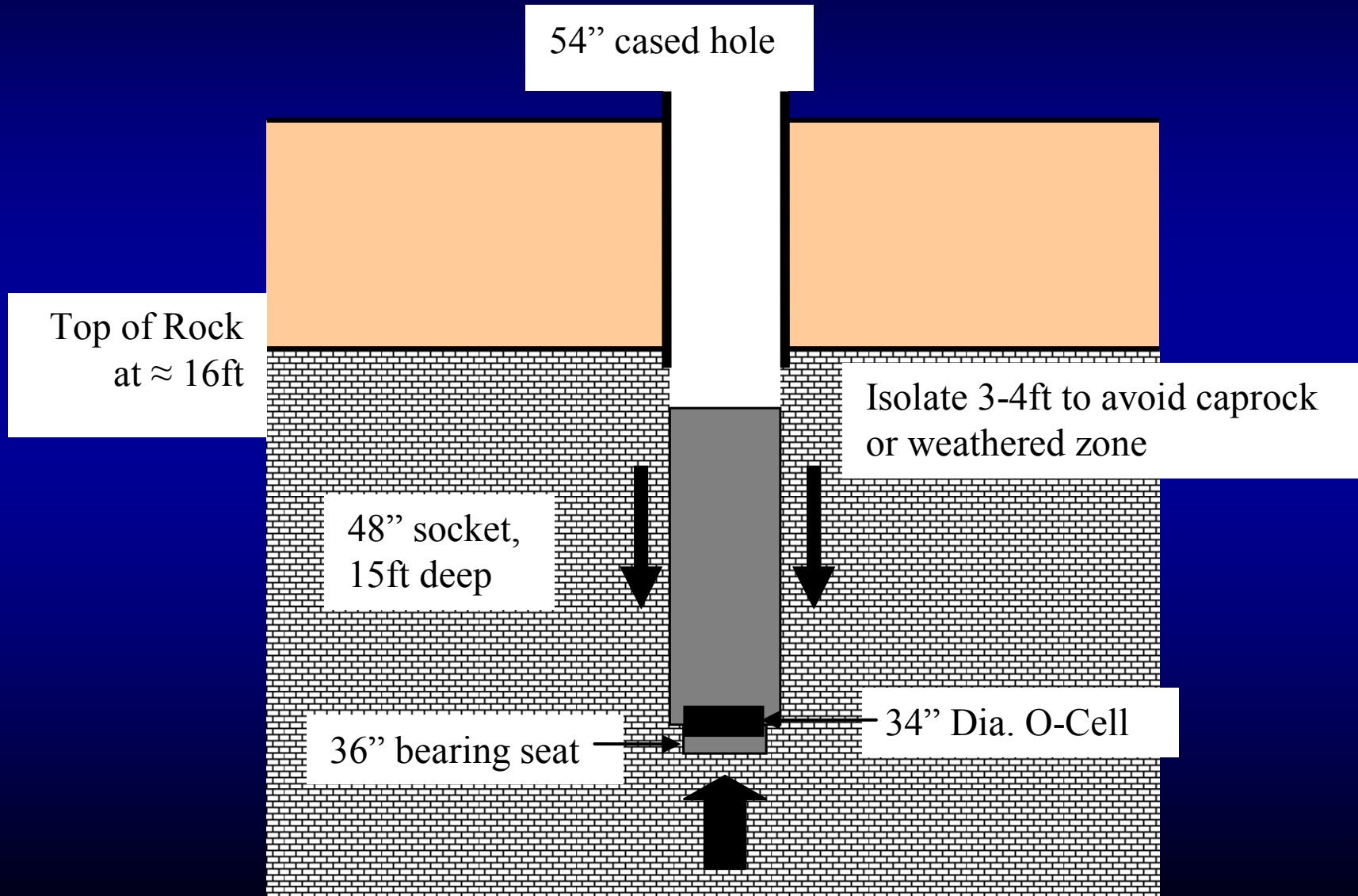
Methodology and Scope

- ◆ Review & evaluate available load test data
- ◆ Perform select load tests under carefully controlled and well defined conditions, representative of local area
- ◆ Involve local practicing engineers to ensure that the tests are representative and meaningful for local practice
- ◆ Evaluate test data and develop recommendations
- ◆ Organize local seminars to transfer research into practice and share local experiences

Test Plan for Nashville

- ◆ Select site with appropriate geologic characteristics
- ◆ Perform two O-cell tests, with machine-only bottom cleaning and less than ideal rock conditions
- ◆ Adjust dimensions of 2nd test based on results of 1st test

1st Test



Boring Logs (thanks PSI)

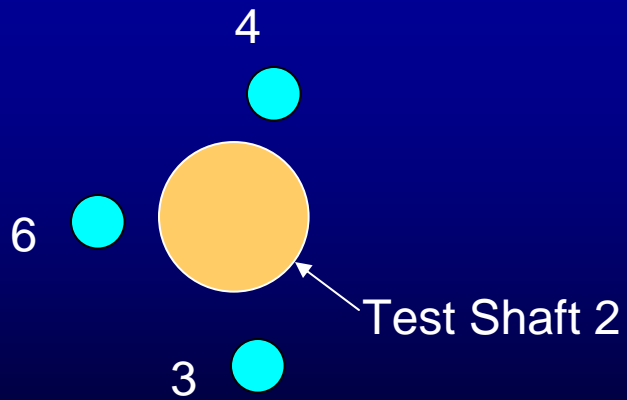
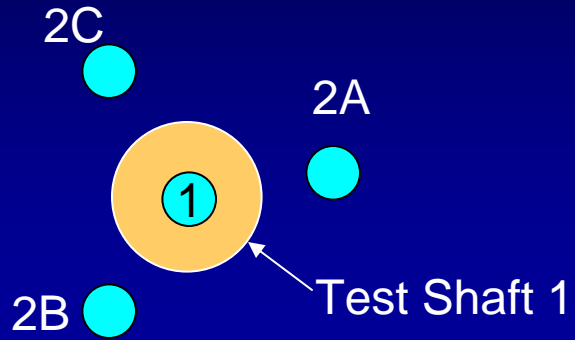
Project: Long Project		PSI No.	Date: 5						
Boring No.: B-1		Total Depth: 36.0'	Elev.						
Boring Method: Hollow Stem Auger		Drill Type: CME-55	Water at Completion of Drilling: After Co						
Elevation (ft)	Depth (ft)	Sample	DESCRIPTION OF MATERIALS	N	N VALUE (pcf)	%W	LL	PI	Q
					10 20 30 40 50 60 70 80 90				
	0.5		6" CRUSHED LIMESTONE Brown CLAY with chert and silt. (DRILLER DESCRIPTION)						
	16.0		Auger Refusal 16.0 Feet; Begin Coring. Light Gray to Gray, Fine to Medium Grained LIMESTONE with clay seams and shale partings, slightly fractured. REC=100%; RQD=64%						
	26.0		Light Gray to Gray, Fine to Medium Grained LIMESTONE with clay seams and shale partings, slightly fractured. REC=100%; RQD=65%						
	36.0		Coring Terminated 36.0 Feet Water Level After Coring 3.0 Feet						

Project: Long Project		PSI No.	Date: 5						
Boring No.: B-2		Total Depth: 36.5'	Elev.						
Boring Method: Hollow Stem Auger		Drill Type: CME-55	Water at						
Elevation (ft)	Depth (ft)	Sample	DESCRIPTION OF MATERIALS	N	N VALUE (pcf)	%W	LL	PI	Q
					10 20 30 40 50 60 70 80 90				
	0.7		6" CRUSHED LIMESTONE Brown CLAY with chert and silt. (DRILLER DESCRIPTION)						
	16.5		Auger Refusal 16.5 Feet; Begin Coring. Light Gray to Gray, Fine to Medium Grained LIMESTONE with clay seams, moderately fractured. REC=93%; RQD=51%						
	26.5		Very Light Gray to Gray, Fine to Medium Grained LIMESTONE with clay seams and shale partings, moderately fractured. REC=97%; RQD=46%						
	36.5		Coring Terminated 36.5 Feet Water Lost During Coring						

Rock Conditions



Borings



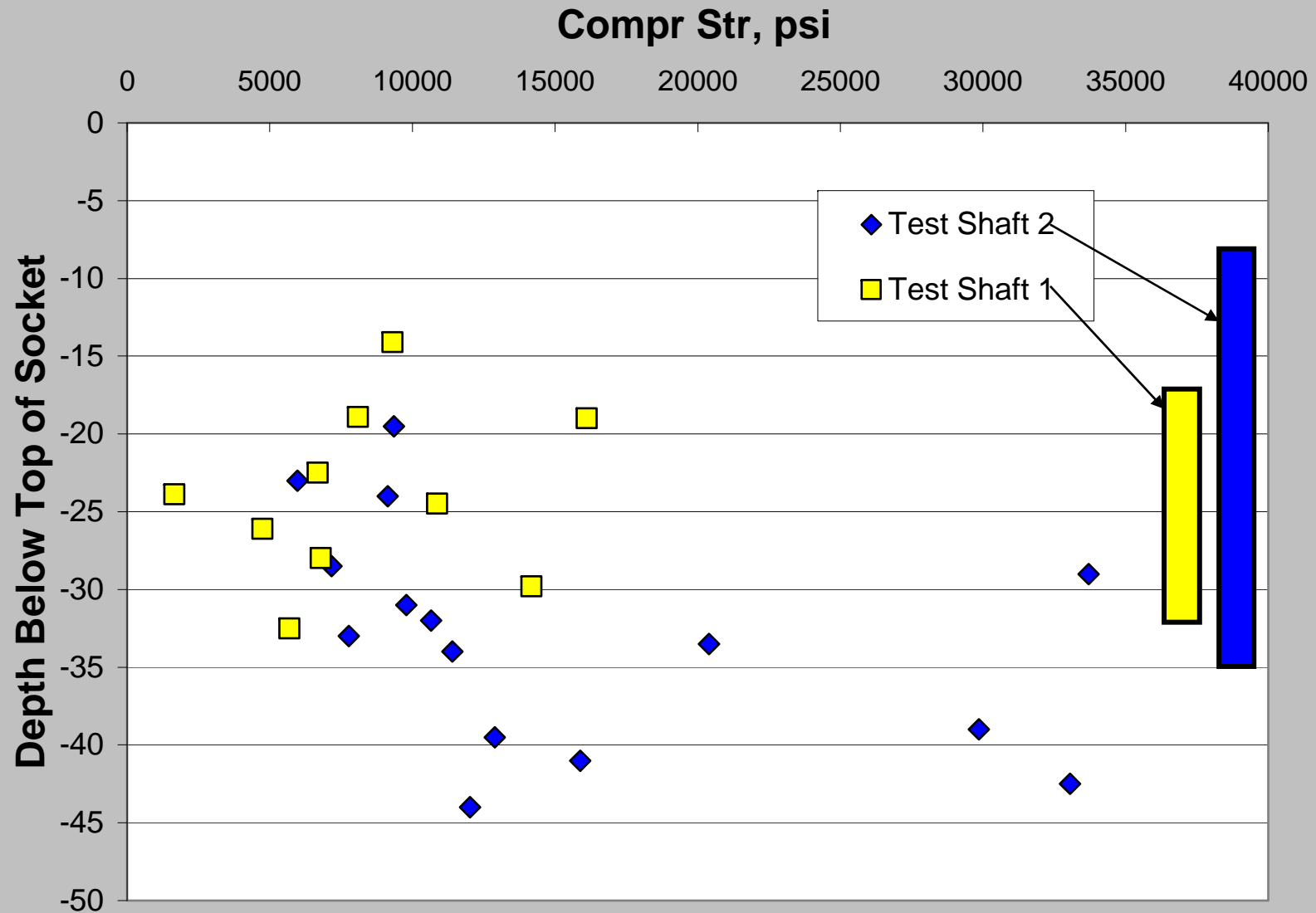
Cores



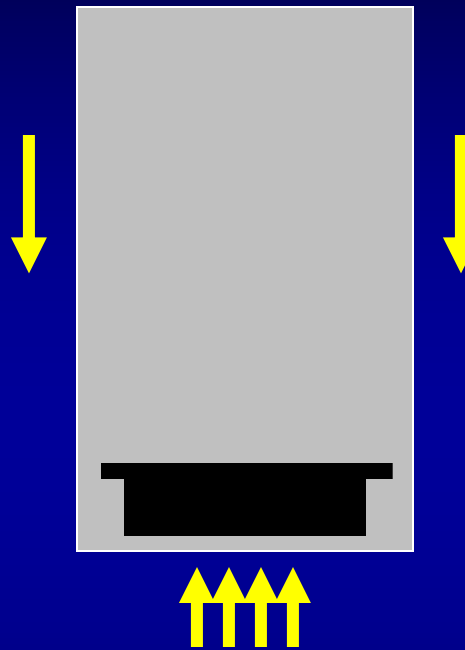
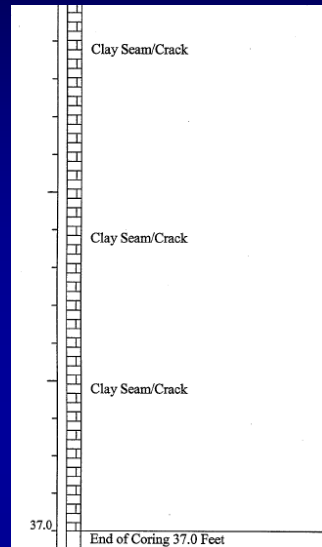
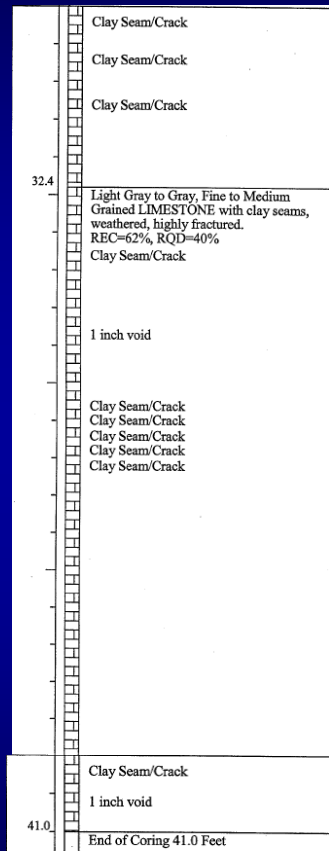
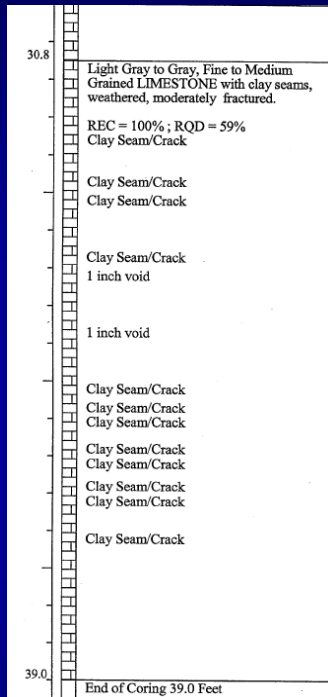
Cores



Core Test Data



Test Shaft 1



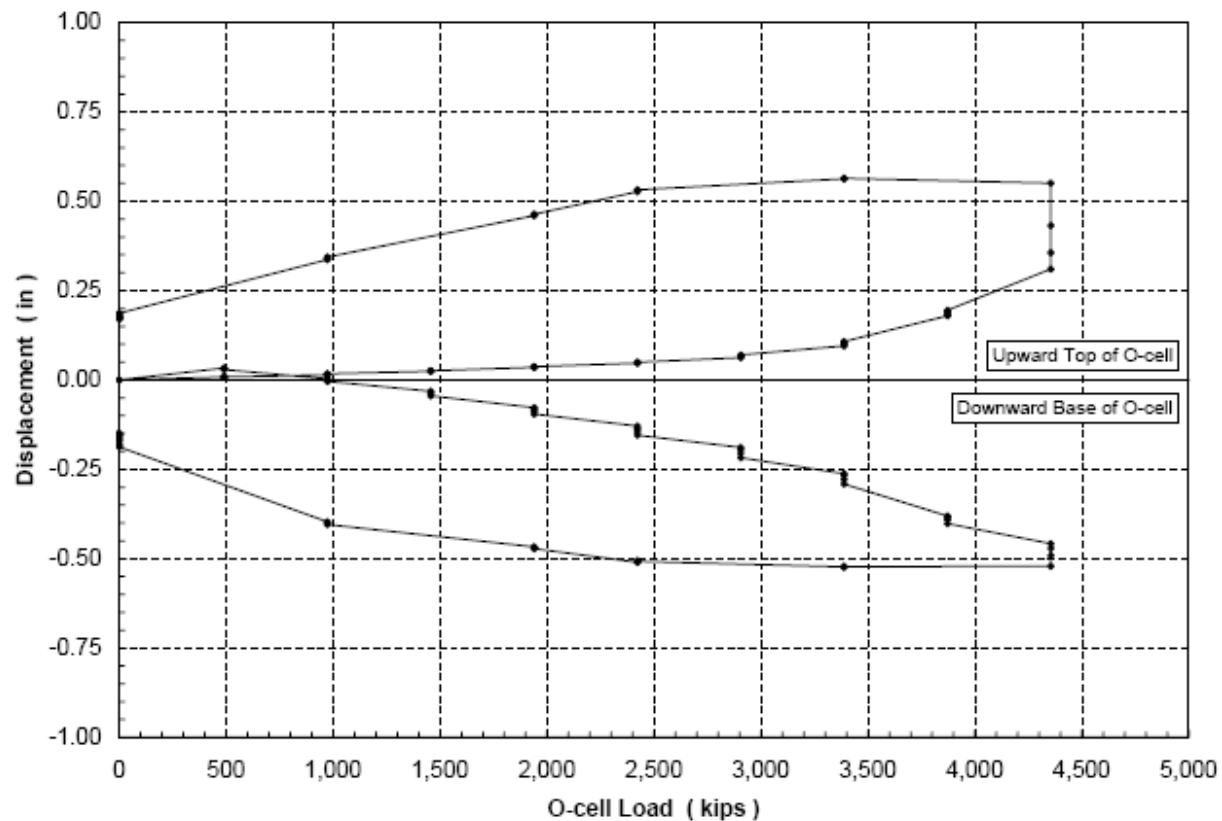
Load Test



Test Results – Shaft 1



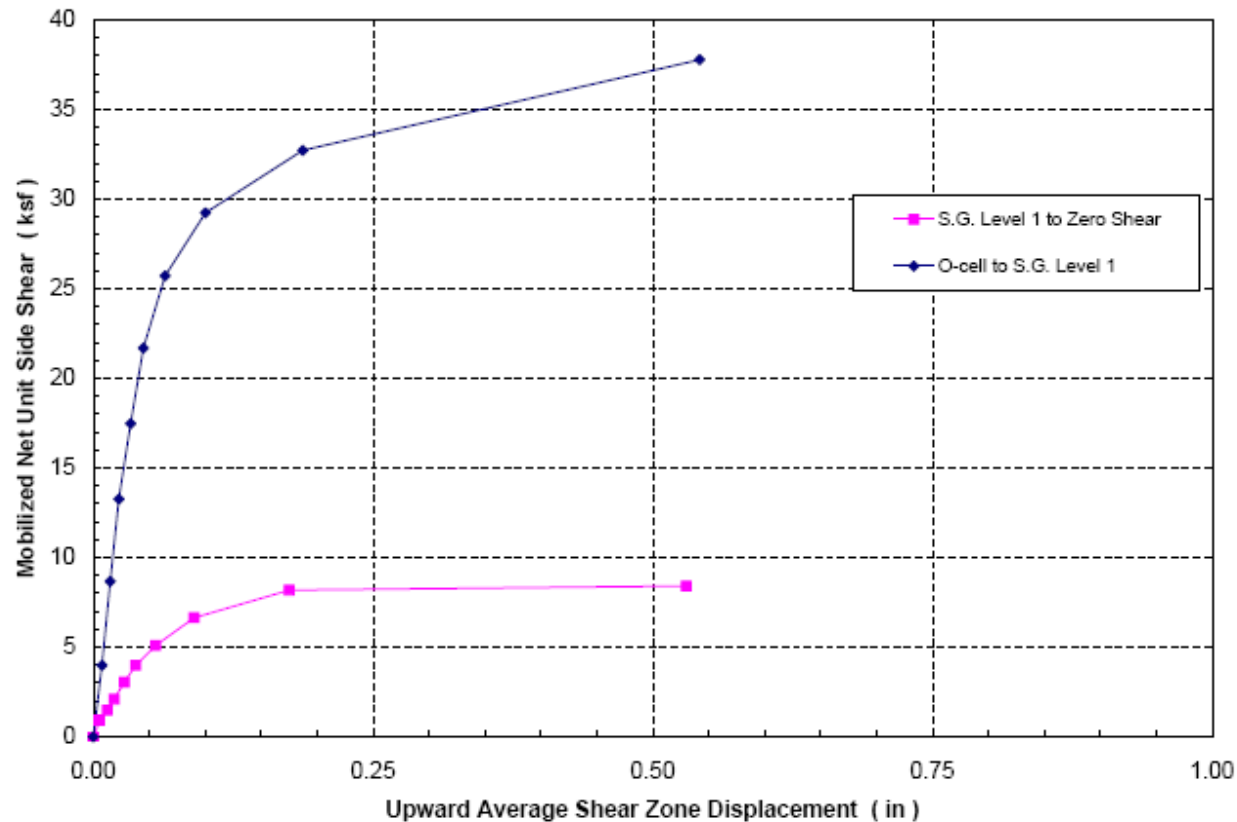
Osterberg Cell Load-Displacement
TS-1 - ADSC Research Project - Nashville, TN



Test Results – Shaft 1



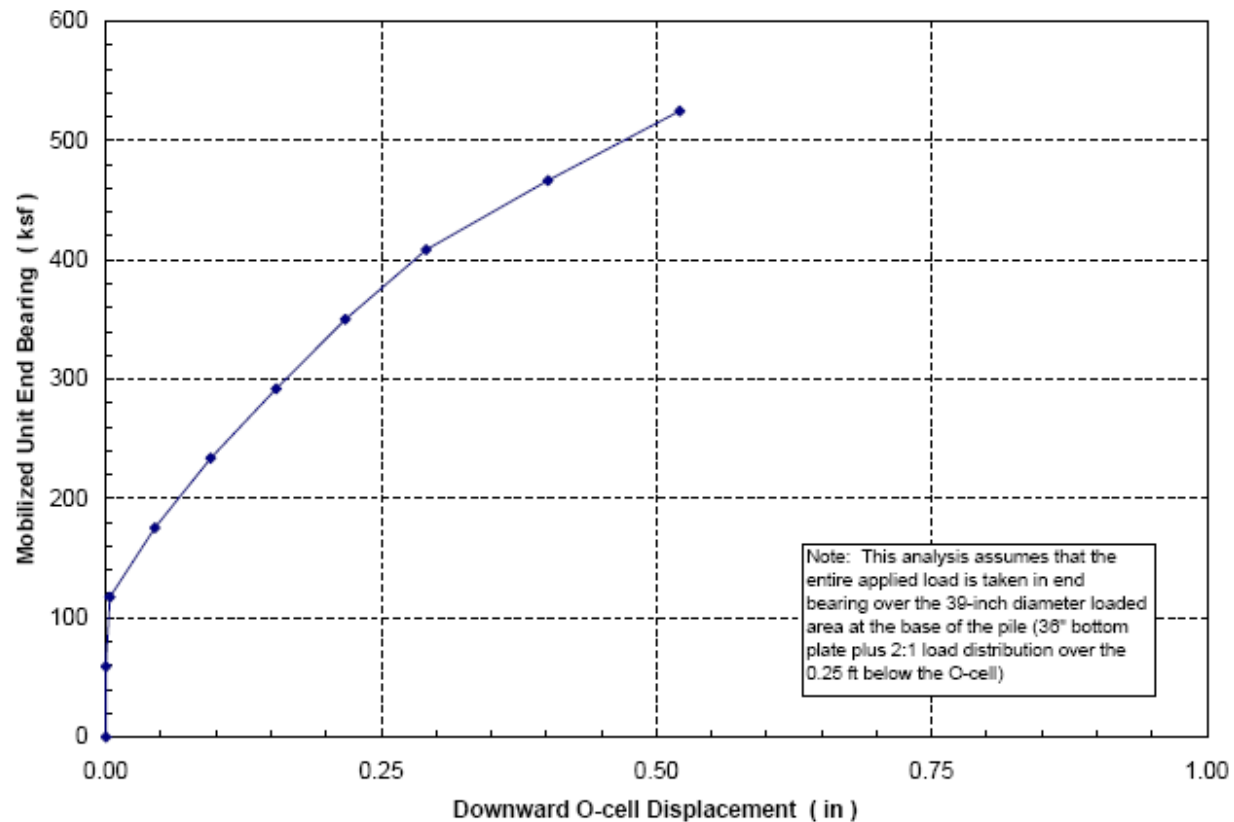
Mobilized Net Unit Side Shear
TS-1 - ADSC Research Project - Nashville, TN



Test Results – Shaft 1



Mobilized Unit End Bearing
TS-1 - ADSC Research Project - Nashville, TN



Predictions

ADSC SE Chapter Research Project - Nashville Test Site Shaft #1

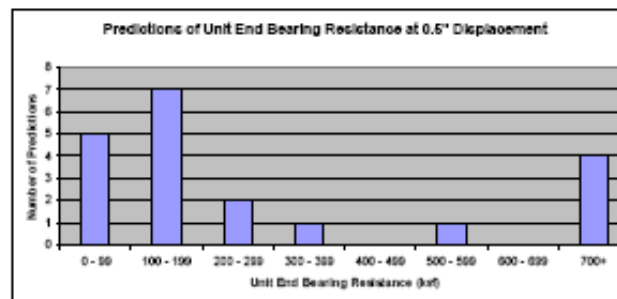
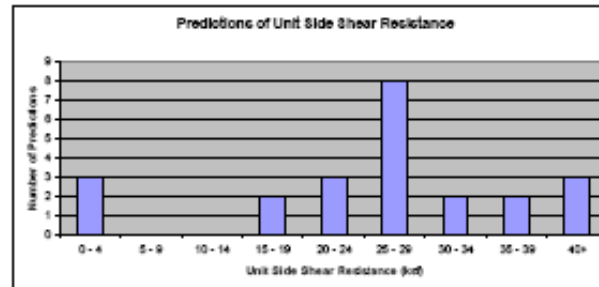
Test Results Summary

Maximum Displacement	0.55	Inch
Mobilized Unit End Bearing at 0.55"	530	ksf
Mobilized Unit End Bearing at 0.5"	520	ksf
Average Mobilized Unit Side Shear	24.2	ksf

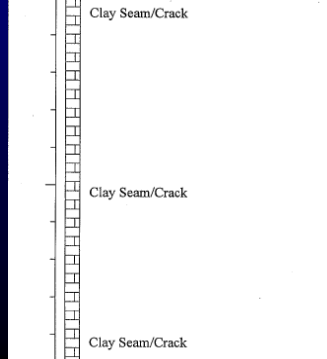
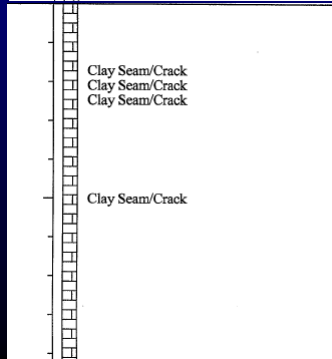
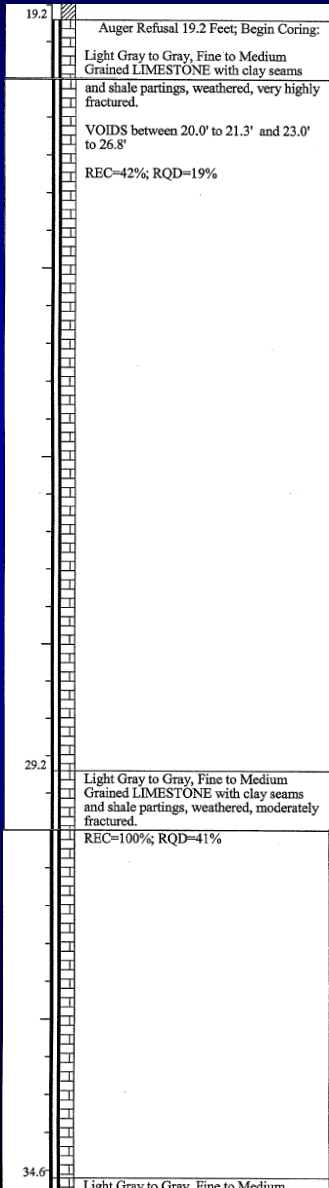
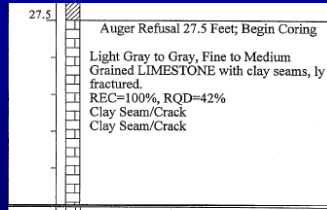
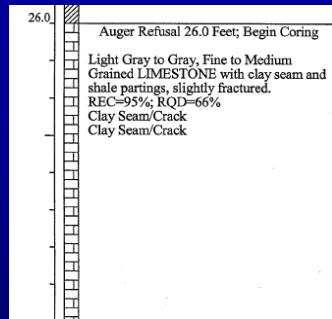
Prediction Analysis

	Max. Unit Base Resistance (ksf)	Max. Unit Side Shear (ksf)	Unit Base Resistance at 0.5' displ. (ksf)
Number of Predictions	20	21	18
Maximum	1039	86.4	987
Minimum	23.5	0.5	2
Mean	332	28	305
Median	170	26	141
Test Results	??	24.2	520

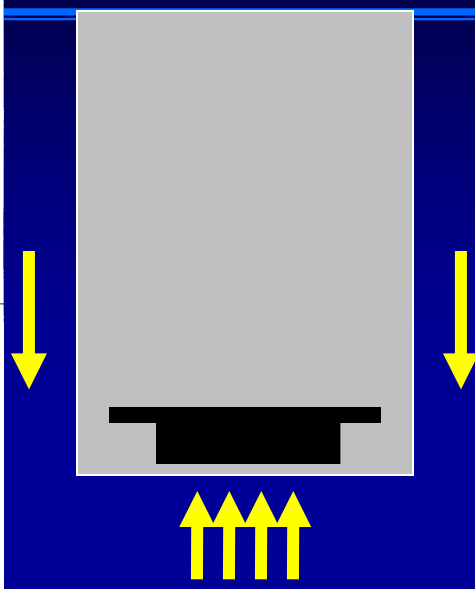
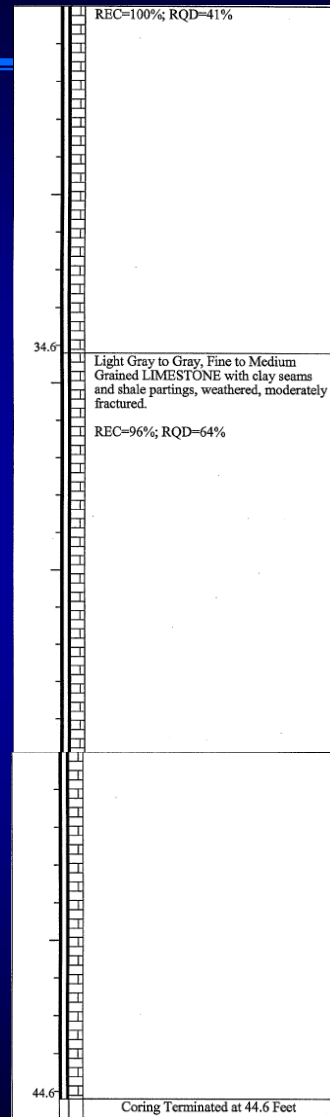
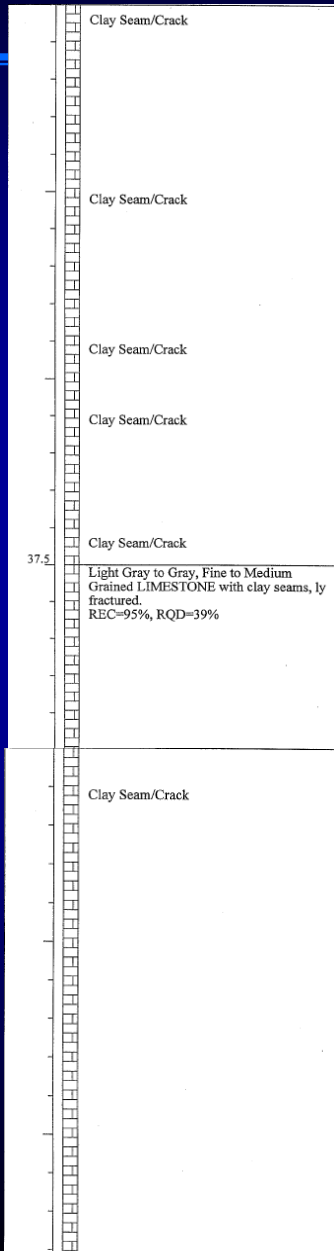
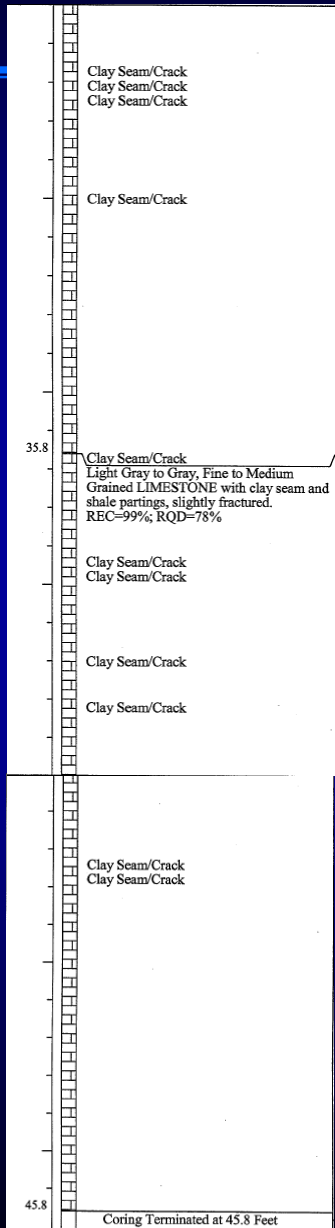
Distributions of Predictions



Test Shaft 2



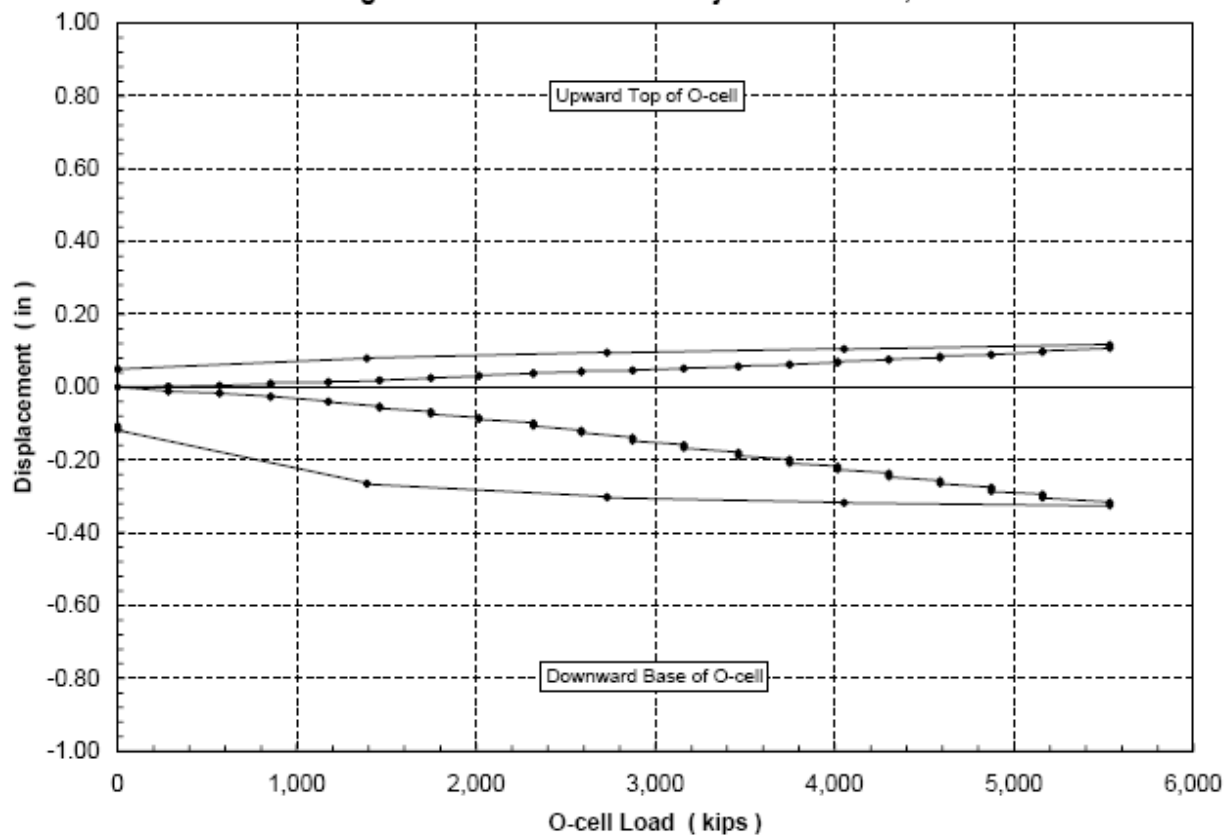
Test Shaft 2



Test Results – Shaft 2



Osterberg Cell Load-Movement
TS-2 - Long Foundation Research Project - Nashville, TN

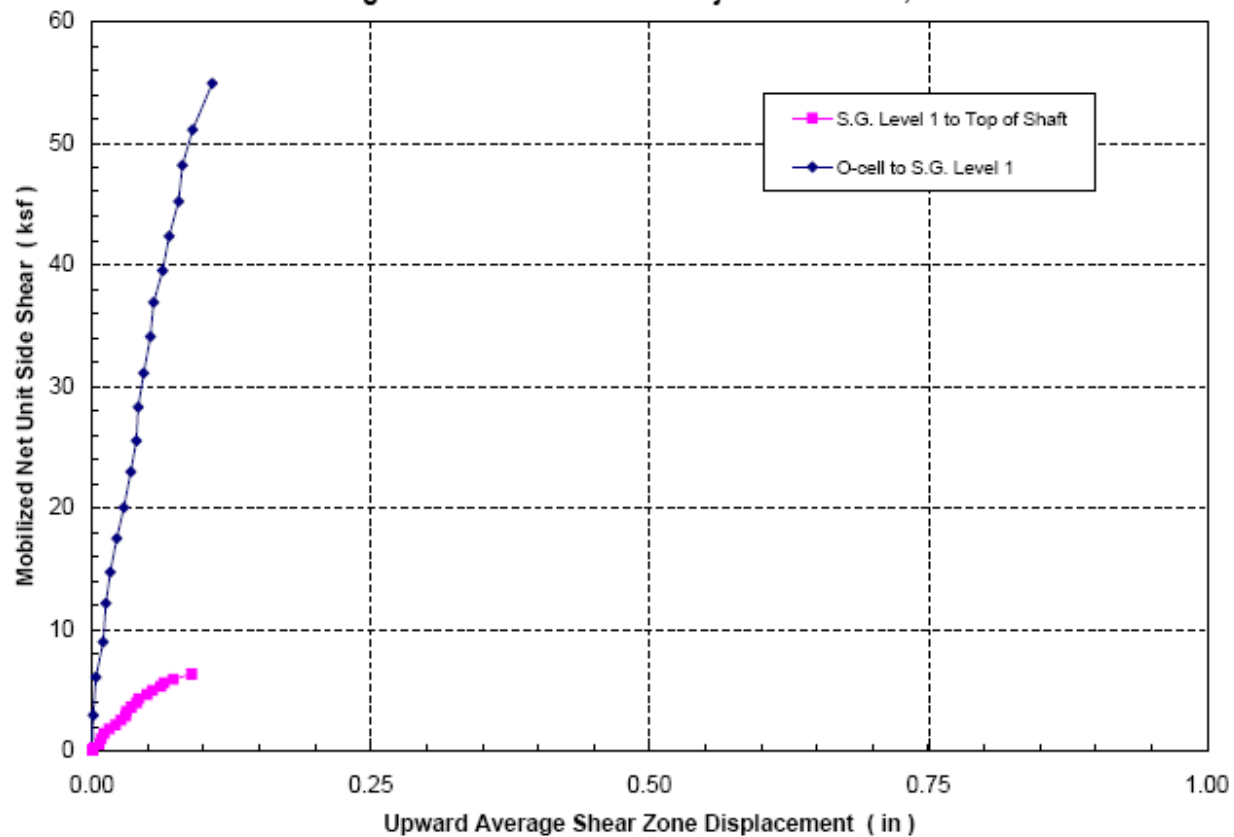


Test Results – Shaft 2



Mobilized Net Unit Side Shear

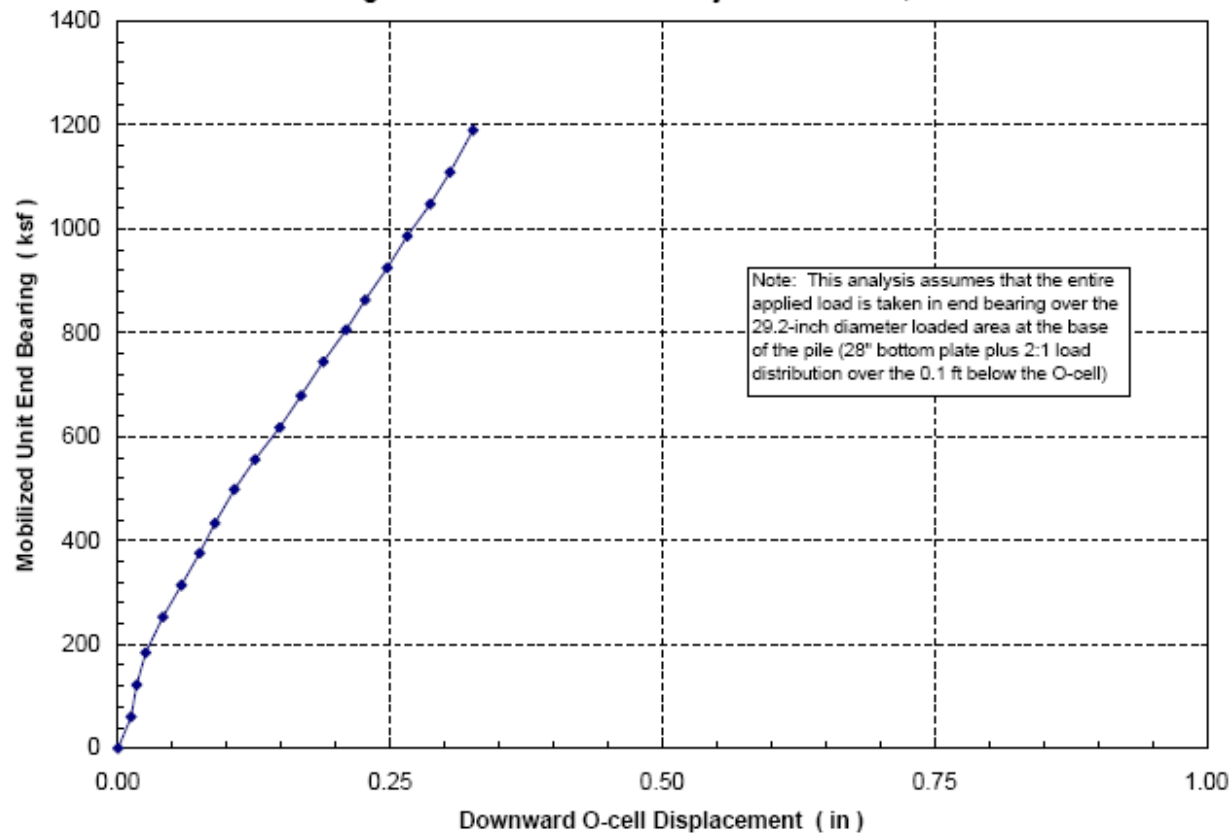
TS-2 - Long Foundation Research Project - Nashville, TN



Test Results – Shaft 2



Mobilized Unit End Bearing
TS-2 - Long Foundation Research Project - Nashville, TN



Conclusions

- ◆ Extremely high end bearing available in “imperfect” rock bearing stratum
- ◆ Unit side shear resistance >25 ksf in limestone with low RQD
- ◆ Machine base cleaning was sufficient

**“A Well-performed Test Is
Worth a Thousand Expert
Opinions.”**